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Geophysical Research Letters

RESEARCH LETTER

10.1029/2019GL082491

Key Points:

- Rivers on lowland deltas have repeated avulsions at a preferential location at the delta apex that scales with the backwater length
- A preferential avulsion node occurs due to flow variability that focuses bed aggradation in the backwater zone
- A preferential node under constant discharge simulations in previous work resulted from assumed initial conditions of uniform bed slope

Supporting Information:Supporting Information S1

Correspondence to:

A. J. Chadwick, achadwick@caltech.edu

Citation:

Chadwick, A. J., Lamb, M. P., Moodie, A. J., Parker, G., & Nittrouer, J. A. (2019). Origin of a preferential avulsion node on lowland river deltas. *Geophysical Research Letters*, 46, 4267–4277. https://doi.org/10.1029/ 2019GL082491

Received 17 FEB 2019 Accepted 31 MAR 2019 Accepted article online 4 APR 2019 Published online 25 APR 2019

TER Origin of a Preferential Avulsion Node on Lowland River Deltas

A. J. Chadwick¹, M. P. Lamb¹, A. J. Moodie², G. Parker³, and J. A. Nittrouer²

¹Division of Geological and Planetary Sciences, California Institute of Technology, Pasadena, CA, USA, ²Department of Earth, Environmental and Planetary Sciences, Rice University, Houston, TX, USA, ³Department of Civil and Environmental Engineering and Department of Geology, University of Illinois at Urbana-Champaign, Urbana, IL, USA

Abstract River deltas are built by cycles of lobe growth and abrupt channel shifts, or avulsions, that occur within the backwater zone of coastal rivers. Previous numerical models differ on the origin of backwater-scaled avulsion nodes and their consistency with experimental data. To unify previous work, we developed a numerical model of delta growth that includes backwater hydrodynamics, river mouth progradation, relative sea level rise, variable flow regimes, and cycles of lobe growth, abandonment, and reoccupation. For parameter space applicable to lowland deltas, we found that flow variability is the primary mechanism to cause persistent avulsion nodes by focusing aggradation within the backwater zone. Backwater-scaled avulsion nodes also occur under less likely scenarios of initially uniform bed slopes or during rapid relative sea level rise and marine transgression. Our findings suggest that flow variability is a fundamental control on long-term delta morphodynamics.

Plain Language Summary River deltas are important for farming and drinking water, human populations, and diverse wildlife. Rivers on deltas are unstable and abruptly change course every 10–1,000 years. These channel shifts are necessary for sustaining coastal landscapes and also pose significant hazards. Here we present a mathematical model that shows how rivers require occasional floods, similar to what is observed on natural rivers, to give rise to a predictable location where rivers shift their course. Model simulations without floods produce rivers that change course at random locations, unlike natural rivers. Our findings resolve differences in previous studies about the importance of floods and illustrate that occasional floods are necessary for natural delta growth.

1. Introduction

Many deltas are built through the deposition of discrete lobes punctuated by lobe-switching events called river avulsions (Jerolmack, 2009; Slingerland & Smith, 2004). River avulsions pose a hazard to human life and property (Kidder & Liu, 2017; Soong & Zhao, 1994) and are fundamental for building new land and nourishing wetland ecosystems (Edmonds et al., 2009; Richards et al., 2002). We need to understand where avulsions occur on lowland deltas to improve predictions of flooding hazards and sustainability.

Deltaic avulsions tend to occur repeatedly at a similar location, termed the avulsion node, which sets the delta apex location and determines delta size (Ganti, Chadwick, Hassenruck-Gudipati, Fuller, & Lamb, 2016; Jerolmack, 2009). The avulsion node on some landforms, typically steeper fan deltas and alluvial fans, is controlled by valley width and slope variations (Blair & McPherson, 1994; Ganti et al., 2014; Hartley et al., 2017), but on lowland deltas avulsion nodes persist on unconfined alluvial plains. The distance from the shoreline to the avulsion node, termed the avulsion length (L_A), typically scales with the backwater length scale of the river (L_b), that is, the ratio of channel depth (H_c) to bed slope (S; i.e., $L_A \sim L_b = \frac{H_c}{S}$, Figure 1a; Chatanantavet et al., 2012; Jerolmack & Swenson, 2007; Paola & Mohrig, 1996). The backwater length approximates the distance that sea level influences river flow upstream and can extend for hundreds of kilometers for low-sloping rivers (Lamb et al., 2012).

The Huanghe, China, provides an example where seven consecutive backwater-scaled avulsions occurred before major engineering (Ganti et al., 2014). In addition, abandoned lobes record six Holocene avulsions on the Mississippi that occurred within the backwater zone (Chatanantavet et al., 2012; Coleman et al., 1998), while two avulsions occurred farther upstream (Chamberlain et al., 2018; Saucier, 1994). Because river avulsions occur infrequently and are difficult to observe directly, flume experiments and numerical

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Figure 1. (a) Correlation between avulsion length (L_A) and backwater length (L_b) from lowland river deltas and backwater-influenced experiments. (b) Planview schematic. Black solid lines are active channel of width *B* within a flood-plain/lobe of width B_f (lobe 3). Broken lines are abandoned channels. After an avulsion, abandoned lobe 4 is reoccupied and its profile is joined with trunk channel at avulsion node (yellow star). (c) Cross-section schematic, showing channel aggradation and floodplain superelevation of the active lobe (lobe 3) relative to the lowest abandoned lobe (lobe 4).

modeling have contributed substantially to our understanding. These modeling studies, however, differ in their explanation for the origin of persistent, backwater-scaled avulsion nodes.

Chatanantavet et al. (2012) hypothesized that avulsion nodes on lowland deltas originate from heightened in channel-bed aggradation in the backwater zone that emerges due to flows of variable discharge. Using quasi-2-D morphodynamic simulations, they showed that low flows deposit sediment in the upstream part of the backwater zone and high flows focus erosion and bypass farther downstream, resulting in a persistent peak in net aggradation in the middle of the backwater zone. Constant discharge simulations, in contrast, yielded quasi-uniform flow and uniform bed aggradation rates. Chatanantavet et al. (2012) did not simulate avulsions and lobe switching, and the river mouth in their study was unrealistically fixed, which prevented riverbed aggradation due to progradation. Nonetheless, subsequent flume experiments that included progradation and natural lobe switching and reactivation supported their hypothesis by showing persistent avulsions about a preferential node at $L_A \sim 0.5L_b$, coinciding with a peak in channel bed aggradation for an experiment with variable flows (Ganti, Chadwick, Hassenruck-Gudipati, Fuller, & Lamb, 2016; Ganti, Chadwick, Hassenruck-Gudipati, & Lamb, 2016). In contrast, a comparable constant discharge experiment did not produce a persistent node, indicating that flow variability is the dominant mechanism to produce backwater-scaled avulsions.

Later numerical modeling studies by Moran et al. (2017) and Ratliff (2017), however, simulated delta growth with river mouth progradation under a range of relative sea level rise rates and constant discharge conditions and found backwater-scaled avulsion nodes despite the lack of flow variability. In their models, a wedge of sediment migrated downstream on a riverbed with an initially uniform slope, and eventually aggradation exceeded an imposed avulsion threshold. Thus, in contrast to experiments (Ganti, Chadwick, Hassenruck-Gudipati, Fuller, & Lamb, 2016; Ganti, Chadwick, Hassenruck-Gudipati, & Lamb, 2016) and earlier models (Chatanantavet et al., 2012), these studies suggest that backwater-scaled avulsions can be produced in models with constant water discharge. However, these models invoked a potentially unrealistic riverbed of uniform slope as an initial condition and measured the potential for avulsion in terms of sediment accumulation thickness relative to the initial topography. In contrast, in natural environments, deltas tend to reoccupy lobes and build over previous fluvio-deltaic deposits. Thus, the assumed initial conditions in these numerical models might affect the emergence of an avulsion node.

Here we aim to elucidate the origin of a preferential avulsion node and unify the contradictory results of previous work. In particular, the model of Chatanantavet et al. (2012) requires flow variability to produce a persistent avulsion node, consistent with available experimental data, but the elimination of river mouth progradation in their model might have biased their results. In contrast, more recent models (Moran et al., 2017; Ratliff, 2017) can produce persistent avulsion nodes with constant discharge, but they impose an unrealistic initial condition. To address these potentially problematic assumptions, we constructed a

quasi-2-D numerical model that allows for repeated lobe construction and avulsion such that lobes build on top of one another, thereby minimizing the role of initial topography as the delta evolves. The model also allows for river mouth progradation. We explored the model behavior over parameter space relevant to natural lowland deltas, including variable flow regimes and relative sea level rise, to identify the conditions that cause a preferential backwater-scaled avulsion node.

2. Methods

We aimed to isolate the cause of preferential avulsion nodes using a simplified model that captures delta lobe construction, avulsion and reoccupation, and river mouth progradation on lowland river deltas. The model does not represent a specific delta. Instead, the framework is generic and includes a deltaic plain with an imposed number of lobes (Figure 1b).

Following previous work, we modeled each lobe as a coupled river and floodplain of uniform floodplain width (B_{f}), channel sinuosity (Ω), wash load ratio (Λ), and bed porosity (λ_p), which is well described by a quasi-two-dimensional mass balance framework (Chatanantavet et al., 2012; Parker, 2004; Parker et al., 2008a, 2008b). Sediment mass balance also incorporates a floodplain representing the active delta lobe extent:

$$\frac{\partial \eta_b}{\partial t} + \sigma = -\frac{(1+\Lambda)\Omega}{(1-\lambda_p)B_f} \frac{\partial Bq_t}{\partial x}$$
(1)

where η_b is channel bed elevation relative to sea level, *t* is time, σ is relative sea level rise rate, *x* is downstream distance, and q_t is width-averaged flux of total bed material load. Sediment is transported in a river of width *B* and deposited uniformly over the floodplain width B_f (Parker, 2004). We routed water using a quasi-2-D backwater equation for water mass and momentum conservation under quasi-steady flow conditions (Chatanantavet et al., 2012),

$$\frac{\mathrm{d}H}{\mathrm{d}x} = \frac{S - S_f}{1 - Fr^2} + \frac{Fr^2}{1 - Fr^2} \frac{H}{B} \frac{\mathrm{d}B}{\mathrm{d}x} \tag{2}$$

where *H* represents the channel depth, *S* is channel bed slope, and $S_f = C_f F r^2$ is friction slope with friction coefficient C_f and Froude number *Fr*. We assumed uniform channel width and a plume with constant spreading angle offshore (Chatanantavet et al., 2012; Lamb et al., 2012), but unlike previous formulations the plume in our model advances and retreats in concert with the river mouth (Text S1 in the supporting information). We routed sediment according to Engelund and Hansen (1967) for total bed material load:

$$q_t = \sqrt{RgD^3} \frac{\alpha}{C_f} \left(\tau^*\right)^n \tag{3}$$

where *R* is submerged specific density of sediment, *g* is gravity, *D* is the median grain size of bed material, τ^* is Shields number, and $\alpha = 0.05$ and n = 2.5 (Engelund & Hansen, 1967). Equations (2) and (3) adequately describe backwater hydrodynamics and sediment transport of sand-bedded rivers (Lamb et al., 2012; Nittrouer et al., 2012; Chatanantavet & Lamb, 2014).

We approximated deltaic evolution over multiple cycles of lobe switching using four one-dimensional profiles of predefined width, representing four distinct lobes (Figures 1b and 1c). Our choice of four lobes is arbitrary but reasonable based on field observations (Chu et al., 2006; Roberts, 1997) and flume experiments (Carlson et al., 2018; Reitz et al., 2010). One delta lobe was active at a given time (Hajek & Edmonds, 2014; Slingerland & Smith, 2004), and the active lobe evolution was governed by equations (1)–(3) and solved using finite differences (Text S2). We varied sediment supply at the upstream end with water discharge such that the normal flow bed slope was held constant, and therefore, erosion and deposition were not driven by changes in sediment supply and water discharge ratios (Paola, 2000). For the delta front, we used a movingboundary formulation following Swenson et al. (2000) and others (Text S1). Inactive lobe shapes were unchanged when abandoned, approximating a river-dominated delta where reworking is minimal (Galloway, 1975); however, abandoned lobes were partially drowned in cases due to relative sea level rise. We used an avulsion criterion given by a critical thickness of aggradation, which we refer to as superelevation ($\Delta \eta$):

$$\Delta \eta(x) \ge H^* H_c \tag{4}$$

in which H_c is the bankfull channel depth and H^* is the avulsion threshold, a dimensionless number that is of order unity (Ganti et al., 2014; Jerolmack & Mohrig, 2007), which we set to $H^* = 0.5$ consistent with field and experimental observations (Ganti et al., 2014; Ganti, Chadwick, Hassenruck-Gudipati, & Lamb, 2016; Mohrig et al., 2000). The critical superelevation $\Delta \eta$ may represent the local floodplain (or levee) elevation relative to the distant floodplain or inactive lobes, or the bed aggradation thickness since the last avulsion (Figure 1c; Ganti et al., 2014; Hajek & Wolinsky, 2012; Mohrig et al., 2000). In our model the floodplain (η_f) aggrades in concert with the channel bed $(\eta_f = \eta_b + H_c;$ Text S1 and Figure 1c) and inactive lobes remain unchanged once abandoned, so both explanations hold. We triggered an avulsion when and where the floodplain elevation of the active lobe exceeded the floodplain elevation of the lowest-elevation abandoned lobe $(\eta_{f,abandoned})$, evaluated at the same distance downstream from the trunk channel:

$$\Delta \eta(x) = \begin{cases} \eta_f(x) - \eta_{f,\text{abandoned}}(x) & \text{for } x \le x_{m,\text{abandoned}} \\ \eta_f(x) - \xi_{\text{sea}} & \text{for } x > x_{m,\text{abandoned}} \end{cases}$$
(5)

where $x_{m,abandoned}$ is the streamwise coordinate of the abandoned lobe shoreline (Figure 1c). Seaward of the abandoned lobe, superelevation is measured relative to sea level (ξ_{sea}), consistent with assumptions in previous work (Ratliff, 2017). The occurrence of extreme floods and hydraulic connectivity with abandoned channels may also affect the location and timing of any one avulsion (Ganti et al., 2014; Nicholas et al., 2018), but following previous work, these effects were neglected in our treatment of multiple avulsion cycles (Hajek & Wolinsky, 2012; Jerolmack & Paola, 2007).

After an avulsion, the river was routed to the lowest abandoned lobe by joining the bed profile of the active channel upstream of the avulsion site (the trunk channel) with the bed profile of the new flow path downstream (the daughter channel; Text S2). This process mimics the tendency of rivers to select steeper paths, fill in topographic lows (Slingerland & Smith, 2004; Straub et al., 2009), and reoccupy previously abandoned channels (Reitz & Jerolmack, 2012). After establishing the new flow path, lobe construction (equations (1)–(3)) and avulsion setup (equation (4)) began anew.

At the start of each model run, the initial state of the riverbed was assumed planar with a uniform downstream slope set to the transport slope for normal flow, similar to previous studies (Chatanantavet et al., 2012; Moran et al., 2017; Ratliff, 2017). However, due to the imposed number of lobes, after four avulsion cycles the river was forced to reoccupy lobes that were previously active. Thus, unlike previous work, the effect of the initial conditions were minimized after the fourth avulsion cycle.

For variable discharge simulations, we implemented flow variability using a log-normal distribution of normal flow depths (Text S3). The distribution is defined by the bankfull exceedance probability F_{bf} , which describes the frequency of overbank flows relative to all possible flows, and the coefficient of variation CV, which describes the magnitude of low flows and high flows relative to the average flow. We randomly sampled the distribution with a characteristic flow event duration (T_e ;Figure S1). Gauge data of monthly mean stage height ($T_e = 1$ month) from several lowland rivers show $F_{bf} \sim 5-20\%$ and $CV \sim 0.2-0.9$ (Table S1; Ganti et al., 2014).

Our simulations explore how deltaic avulsion patterns respond to variable river discharge, relative sea level rise, and initial topography by systematically varying the discharge and sea level parameters for a base case characteristic of large, low-sloping deltas. We nondimensionalized the model so that it can be applied to a wide range of river conditions (Text S4). The model is governed by nine input dimensionless parameters: bankfull Froude number in the normal flow reach ($Fr_{n,bf}$), bankfull Shields number in the normal flow reach ($\tau^*_{n,bf}$), friction factor (C_f), offshore basin floor depth normalized by bankfull depth (H^*_b), time normalized by the channel adjustment timescale ($t^* \sim \frac{tq_t}{L_bH_c}$), a dimensionless rate of relative sea level rise ($\sigma^* \sim \frac{\sigma L_b}{q_t}$), and the flow variability parameters (F_{bf} , CV, T^*_e ; Text S3), where $T^*_e = \frac{T_eq_t}{L_bH_c}$ is a dimensionless flow duration.

In all simulations presented here, we assumed constant values typical of large sand-bedded rivers where $Fr_{n,bf} = 0.17$, $\tau_{n,bf}^* = 1$, $C_f = 0.005$, $H_b^* = 2$ (Table S1) and changed only flow variability parameters (F_{bf} , CV, and T_e^*) and relative sea level rise (σ^*). Model sensitivity to the other parameters is discussed in Text S6. For each set of dimensionless parameters, simulations proceeded until 13 avulsions occurred, which was sufficient to capture trends in avulsion location (Text S4).

3. Avulsion Nodes Originating From Initial Conditions and Flow Variability

We first considered a scenario of constant river discharge equal to the bankfull condition and constant relative sea level ($F_{bf} = 1, CV = 0, \sigma^* = 0$), with other model parameters set to the base case. During the first four avulsion cycles, the delta built lobes on the initial surface, which was a plane with a uniform seaward slope. The first avulsion occurred after 1.8 normalized time ($\Delta t^* = 1.8$), equivalent to 9–720 years for a range of parameters typical of natural deltas (Table S1 and Text S4), with an avulsion length equal to $0.78L_b$ ($L_A^* = 0.78$; Figure 2a). Avulsion lengths were similar for the second, third, and fourth avulsions ($L_A^* = 0.74, 0.68, 0.98$, respectively). In contrast, after avulsion cycle four the constant discharge delta built upon previously abandoned delta lobes and did not produce a backwater-scaled avulsion node. Normalized avulsion lengths varied considerably for these later avulsions ($L_A^* = 0.79-8.3$), and when avulsions occurred a reach of $3-5L_b$ was within 10% of the avulsion threshold, indicating no dominant avulsion location.

The consistent avulsion length during the first four avulsion cycles was a consequence of the assumed initial bed topography, and not due to backwater hydrodynamics. Delta front progradation led to channel aggradation and a quasi-steady concave-up bed elevation profile (Bijkerk et al., 2016; Muto & Swenson, 2005), in contrast to the uniform slope bed profiles that were assumed for all lobes as initial conditions. Differencing the concave-up active lobe profile from the uniform slope of the lowest inactive (and yet to be active) lobe profile resulted in a systematic downstream increase in superelevation (Figure 2b). Therefore, avulsions occurred at the farthest downstream location that was allowed, where superelevation was greatest, equivalent to the shoreline location on the inactive lobe elevation approached sea level and thus superelevation approached zero (equation (5)). Avulsions that occurred at the shoreline of abandoned lobes necessarily scaled with the backwater length due to geometry; lobes prograded a unit fraction of the backwater length-scale before avulsing (i.e., the lobe progradation distance *D* scales as $D \approx H^*H_c/S \approx H^*L_b$; Ganti et al., 2014).

After avulsion cycles 1–4 in the constant discharge model, the delta completely reworked its initial uniform slope and superelevation therefore was assessed by comparing the active lobe to previously occupied lobes, rather than to the planar initial surface. River profiles in these later avulsion cycles prograded with a quasi-steady and self-similar shape, causing nearly uniform deposition and a similar likelihood of avulsions everywhere, including far outside of the backwater zone (Figure 2c). Thus, avulsion locations and their apparent scaling with the backwater length in cycles 1–4 were a geometric artifact resulting from the assumed initial bed topography. Four avulsions were required to rework the initial condition because four delta lobes were imposed (Figure 1a and Text S6). In absence of the uniform slope initial condition, constant discharge conditions did not produce a persistent backwater-scaled avulsion node.

Next, we considered a model run identical to the constant discharge case but with variable discharge, using CV = 0.53, $F_{bf} = 0.05$, and $T_e^* = 0.001$, which is typical of lowland rivers (Table S1). In this case, we observed a preferential avulsion node corresponding to an avulsion length nearly equal to the backwater length scale, $L_A^* = L_A/L_b \sim 1$ (Figure 2d), which persisted through many avulsion cycles even after there was no longer an influence from the planar initial surface (Figures 2e and 2f). Consistent with previous studies (Chatanantavet et al., 2012; Ganti, Chadwick, Hassenruck-Gudipati, Fuller, & Lamb, 2016; Lamb et al., 2012), periods of low flow had enhanced deposition due to spatial deceleration through the backwater zone and high flows eroded the downstream-most reach, resulting in a spatial peak in deposition rate midway through the backwater zone when averaged over many flow events. The avulsion node was coincident with the location of maximum deposition rate, and only a short reach (<0.4L_b) was within 10% of the threshold at the time of an avulsion. Outliers are due to major avulsions that shifted the avulsion node downstream once



Distance downstream normalized by backwater length-scale, $x^* = x/L_{b}$

Figure 2. Model results for avulsion length through time over 13 avulsion cycles under constant discharge (a) and variable discharge (d) with parameters set to base case $(Fr_{n,bf} = 0.17, \tau_{n,bf}^* = 1, C_f = 0.005, H_b^* = 2)$. Red error bars indicate portion of reach within 10% of threshold superelevation necessary for avulsion. Outliers in cycles 8 and 11 of the variable discharge case are due to transient long-profile adjustment following major avulsions (cycles 7 and 10) that shift the avulsion node seaward and aggrade the trunk channel (Text S5). Results for channel long-profile under constant discharge conditions (b and c) and variable flows (e and f) for two example avulsion cycles affected (cycle 1) and unaffected (cycle 5) by initial conditions. Black lines are riverbed profile at start (dashed) and end (solid) of an avulsion cycle. Floodplain profiles of active lobe (gray solid line) and lowest inactive lobe (gray dashed line) are used to calculate superelevation (see inset). Downstream of inactive lobe shoreline location (red circle), floodplain superelevation is measured relative to sea level. Black triangles are river mouth at end of the avulsion cycle. Yellow stars show avulsion location. Delta progradation extended model domain length by ~4L_b over 13 cycles, leading to an increase in maximum possible avulsion length.

the four lateral lobes were built, causing trunk channel aggradation and overall shoreline progradation (Text S5 and Figure S2).

4. Necessary Degree of Flow Variability

Given the importance of variable flows in controlling avulsion location for deltas that lack uniform bed slopes as initial conditions, we quantified how much flow variability is necessary to drive a preferential avulsion node. We ran 21 numerical experiments to systematically vary the coefficient of flow variation (CV), bankfull exceedance probability (F_{bf}), and dimensionless flow duration (T_e^*) within a parameter space that represents many natural rivers (Table S1). For each model run we changed one of these parameters and held all other parameters to base case values. We focused our analysis on cycles 5–13 that were not affected by the initial uniform bed slope.

Isolation of the coefficient of variation (CV) reveals that there is an intermediate range 0.1 < CV < 0.6 where modeled deltas preferentially avulsed within the upstream half of the backwater zone (Figure 3a). Similarly, avulsions occurred at a preferential node so long as less than 5% of flows exceeded bankfull ($F_{bf} < 0.05$;

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Figure 3. Model results for avulsion length with changing flow variability parameters: CV(a), $F_{bf}(b)$, and $T_{e}^{*}(c)$, as well as variation of relative sea level rise rate σ^{*} under constant discharge (d) and variable discharge (e). Black circles show avulsion locations averaged over cycles not influenced by initial conditions (cycles 5–13), and gray shaded areas denote the average reach within 10% of avulsion threshold at times of avulsion.

Figure 3b) and that the duration of flow events was less than 10% of the reach-filling timescale ($T_e^* < 0.1$; Figure 3c). The conditions needed for a preferential avulsion node predicted by the model are common for natural lowland rivers (Table S1) and correspond to a state of continuous riverbed adjustment, where the scours cut by large floods are partially filled during intervening low flows. Continuous riverbed adjustment is necessary for persistent backwater effects (Chatanantavet & Lamb, 2014) and in our model resulted in broad convex-up portions of the long profile when averaged over many flood events (Figure S3). Under milder flow regimes (CV < 0.1) and longer flow durations ($T_e^* > 0.1$) the riverbed fully adjusted to normal flow conditions, resulting in uniform aggradation rates without a preferential avulsion location similar to the constant discharge scenario. Flashier flow regimes (CV > 0.6) and high probabilities of bankfull exceedance ($F_{bf} > 0.05$) also lacked a preferred avulsion node, as the riverbed adjusted to normal flow conditions associated with large floods.

5. Avulsion Nodes Originating From Relative Sea Level Rise

In previous sections, the dimensionless relative sea level rise rate was set to 0 to isolate the initial conditions and flow variability. To relax this assumption, we varied the dimensionless relative sea level rise rate across a range that encompasses many modern deltas ($\sigma^* = 10^{-3} - 10^0$) under constant discharge (Figure 3d) and variable discharge (Figure 3e) conditions, with other parameters identical to the base case (Table S1). Similar to the scenario of $\sigma^* = 0$, constant discharge deltas with $\sigma^* = 10^{-3} - 10^{-1}$ did not have a preferential node, whereas all variable discharge cases had a preferential avulsion node with $L_A/L_b\sim$ 1. Thus, relative sea level rise at moderate rates common to modern deltas did not significantly affect avulsion node occurrence (Figures 3d and 3e). However, at very high rise rates ($\sigma^* > 10^{-1}$) the river mouth retreated upstream, forcing a strong downstream increase in deposition. Thus, for $\sigma^* > 10^{-1}$ the downstream increase in aggradation resulted in avulsion locations that coincided with the shoreline of the lowest-elevation inactive lobe, which scaled with the backwater length for the same geometric reasons as in the cases with planar initial conditions (Figure S4).

6. Discussion and Conclusions

Our results reconcile previous work by showing that variable flow regimes are necessary to produce backwater-scaled avulsion nodes on lowland deltas. Our finding is consistent with experiments that isolated the role of flow variability (Ganti, Chadwick, Hassenruck-Gudipati, Fuller, & Lamb, 2016) on deltas that experienced continuous lobe growth, abandonment, and reoccupation. A certain amount of flow variability $(0.1 < CV < 0.6, F_{bf} < 0.05, T_e^* < 0.1)$ is necessary to produce persistent backwater effects so that the riverbed is in a continual state of morphodynamic adjustment (Chatanantavet & Lamb, 2014). These conditions are common to natural rivers (Table S1). Continual bed adjustment from floods in our model produced very broad, low-relief upward convexities in the riverbed in the backwater zone (Figures 2e, 2f, and S3), consistent with the bed topography of the lower 200–700 km of the Mississippi (Harmar, 2004; Nittrouer et al., 2012; Figure S5), which may be a topographic signature of backwater-mediated avulsions in other rivers.

In contrast, constant discharge numerical experiments tend toward a graded state without strong backwater effects or a preferential node location. Our results suggest that previous numerical models that lacked variable discharges and produced backwater-scaled avulsions (Moran et al., 2017; Ratliff, 2017) were likely affected by initial conditions of uniformly sloped initial surfaces. A backwater-scaled avulsion node under these conditions is a geometric consequence of assessing superelevation of a prograding channel or lobe relative to a planar seaward sloping landscape. For similar reasons, relative sea level rise can also cause persistent avulsion nodes under constant discharge conditions, but only under high rise rates that cause marine transgression. In these cases, avulsions occur at the most downstream location allowed—near the inactive lobe shoreline, which was $\sim L_b$ upstream of the active river mouth at the time of avulsion due to the geometry of lobe progradation. Although the simulations of Chatanantavet et al. (2012) also had an initial uniform bed slope and relative sea level rise, they did not produce a persistent avulsion node under constant discharge conditions because the model lacked river mouth progradation.

In a sensitivity analysis, we found that changing other model parameters does not affect our results on the origin of preferred avulsion node (Text S6). Larger avulsion thresholds, H^* , cause avulsions to occur farther upstream but do not change the overall results (Figure S6). Likewise, the number of imposed delta lobes affects the number of avulsion cycles that are affected by the initial conditions, and the frequency of trunk channel filling avulsions (Text S5), but does not affect the origin of a preferential avulsion node.

Our variable discharge simulations produced avulsion lengths within a factor of 2 of the backwater length scale, similar to the distribution of avulsion lengths on the Huanghe (Ganti et al., 2014) and Mississippi (Chatanantavet et al., 2012; Coleman et al., 1998). Following four lateral avulsions that occupied the available prograding lobes, the avulsion node in our model shifted downstream in tandem with net shoreline progradation, as has been documented on the Huanghe (Ganti et al., 2014) and in flume experiments (Ganti, Chadwick, Hassenruck-Gudipati, Fuller, & Lamb, 2016). Our model also produced outliers in the avulsion length distribution, where deltas with a backwater-scaled avulsion node sometimes have avulsions such farther upstream, similar to the Mississippi (Chamberlain et al., 2018). These larger-scale avulsions occurred in our model near the time when the avulsion node shifted downstream; once a full set of lateral avulsions occurred, the trunk channel upstream of the avulsion node must aggrade to allow continued net progradation (Text S5).

Avulsion node locations could be different when channels reoccupy topographically dissimilar lobes (e.g., the Danube; Giosan et al., 2005), cut new channels in the floodplain (Hajek & Edmonds, 2014), or where lobes have been modified by marine processes (e.g., the Red River; Mathers & Zalasiewicz, 1999). However, when channels build upon topographically similar abandoned delta lobe topography, which is a common scenario, a backwater-scaled avulsion node emerges when flow variability is sufficient to cause a peak in aggradation within the backwater zone. Thus, changing flow regimes due to climate or built infrastructure may affect flood hazards and wetland sustainability by shifting the location of future avulsions.

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Acknowledgments

We acknowledge National Science Foundation grant EAR 1427262 and the Resnick Sustainability Institute at Caltech for support. We thank Vamsi Ganti, Gail Kineke, Ben Hobbs, Hongbo Ma, Brandee Carlson, Kensuke Naito, and Lisa Kumpf for insightful discussions and Elizabeth Hajek and Wonsuck Kim for constructive reviews. Data are available at the website (http://sead-published.ncsa.illinois. edu/seadrepository/api/researchobjects /urn:uuid:5c37c889e4b0a8e144f6565f). Code is available at the GitHub (https:// github.com/achadwick2323/Origin-ofa-preferential-avulsion-node-onlowland-river-deltas).

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